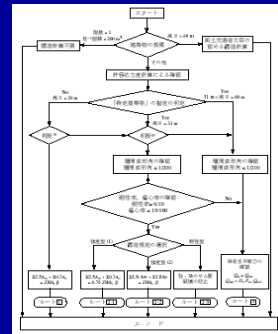


## Seismic Code in Japanese Building Standard Law

## Requirement for calculation

- No calculation
  - Single story, <200m<sup>2</sup>
- Conventional method
  - Allowable stress design
  - Ultimate strength design
    - ✓ Root1
    - ✓ Root2-1,2-2,2-3
    - ✓ Root3
- New method (2000-)
  - Capacity spectrum method
- Dynamic analysis
  - >60m height



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## Conventional method

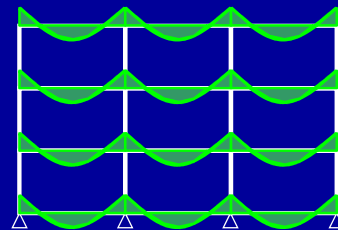
- 1<sup>st</sup> step (allowable stress design)
  - Long-term load = dead load + live load
    - ➔ serviceability limit state
  - Short-term load =
    - L. load + seismic load (moderate)
    - ➔ serviceability & reparability limit state
- 2<sup>nd</sup> step (ultimate strength)
  - = L. load + seismic load (large)
  - ➔ safety & ultimate limit state

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## Stress by vertical load (F.E.M. in beams)



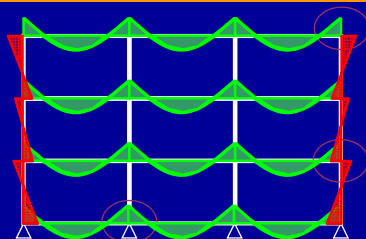
- Flexural moment at beam ends is transferred to columns due to the balance of Flexural moment of beams and columns at a node.
  - Generally, moment in an interior column is 0 or small, because of balance of moments in beams at both side.
  - Moment at an exterior end of a beam is transferred to the connecting exterior column.

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## Stress by vertical load (M. in columns)



M in column is equal to M at beam end.

to M at beam end, is divided to upper and lower columns.

If M at left and right beam ends are balanced, M in column is 0.

- M at beam ends are not same with F.E.M., because beam ends are not completely fixed. However, general tendency can be obtained by the method shown above.
- Accurate result can be obtained by slope deflection method and/or moment distribution method

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## Story shear Qi and coefficient Ci

$$C_i = Z \cdot R_t \cdot A_i \cdot C_0, \quad Q_i = C_i \cdot W_i$$

- Z : zone factor (0.7-1.0)
- R<sub>t</sub> : vibration characteristic fac. (1.0-)
- A<sub>i</sub> : story shear distribution for i-th story (1.0-)
- C<sub>0</sub> : base shear coef.
  - ✓ 0.2 1<sup>st</sup> step (moderate EQ)
  - ✓ 1.0 2<sup>nd</sup> step (large EQ)
- W<sub>i</sub> : weight of buildings i-th and upper stories

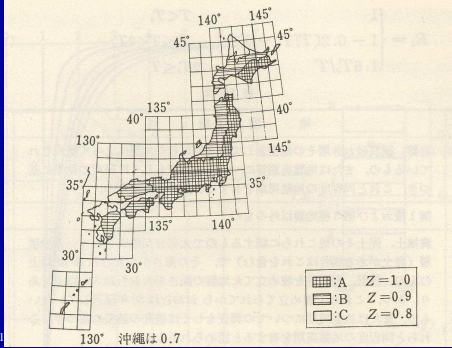
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## Zone factor Z

- A factor for the level of seismic activity at construction site



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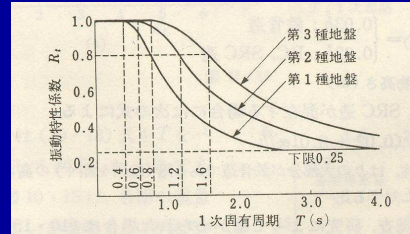
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## Vibration characteristic fac. $R_t$

- A factor for vibration period of building and soil type

$$R_t = 1 - 0.2 \left( \frac{T}{T_c} - 1 \right)^2$$

Period T of RC buildings  
 $T = 0.02H(\text{sec})$  H: height(m)



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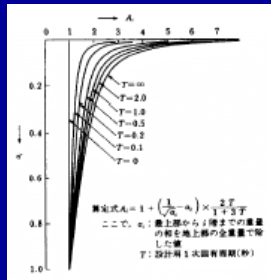
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## Story shear distribution $A_i$

- A factor for story shear distribution along the height of a building

$$A_i = 1 + \left( \frac{1}{\sqrt{\alpha_i}} - \alpha_i \right) \frac{2T}{1 + 3T}$$

$$\alpha_i = W_i / W_1$$



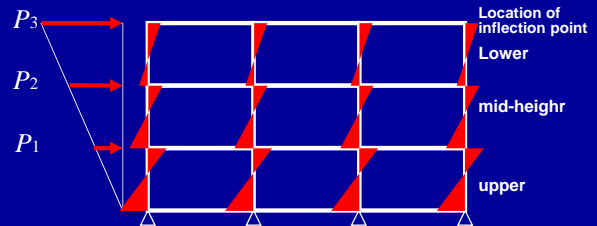
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## Stress due to seismic force in column

- Story shear is distributed in columns.
- Larger stress in lower story.



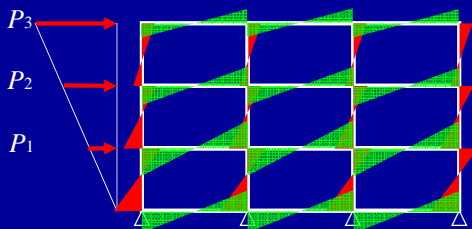
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## Stress due to seismic force in beam

- Flexural moment in column is transferred to beams.
  - Summation of moment in upper and lower columns at a node is divided to left and right beams.
- Stress for short-term load is
  - stress for long-term load + stress for seismic force



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## Cal. for seismic force (2<sup>nd</sup> step)

- Root 1 (<20m)
  - Approximated strength by column and wall section area
 
$$0.7A_c + 2.5A_w \geq ZWA_i$$
- Root 2 (<31m)
  - 2-1, 2-2: Approx. strength (similar with Root1)
  - 2-3: ensure ductility (prevent shear failure)
- Root 3
  - Cal. of ultimate lateral strength

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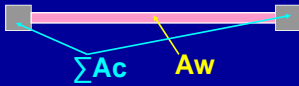
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## Root 1 (approximated evaluation)

$$0.7\sum A_c + 2.5\sum A_w \geq Z W A_i$$

- Column shear strength =  $0.7N/mm^2 \times$  total sectional area( $mm^2$ )  
average ultimate shear stress
- Wall shear strength =  $2.5N/mm^2 \times$  total sectional area( $mm^2$ )



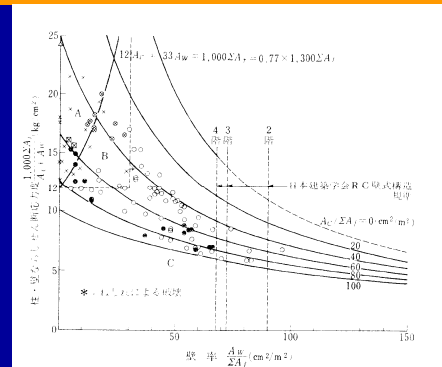
- Design seismic force =  
building weight  $\times 1.0g$  (acceleration response) =  $\sum W$   
(assuming  $D_s=1$ )

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## Shiga's diagram



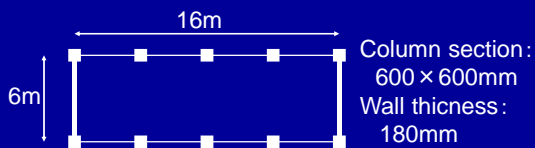
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## example

evaluate seismic safety by the equation below for a 3-storied RC building structure with a floor plan shown in the figure below.



$$0.7\sum A_c + 2.5\sum A_w \geq \sum W$$

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## Answer 1

- Total weight of building  $\sum W$   
 $12kN/m^2 \times 6m \times 16m \times 3\text{story} = 3456 \text{ kN}$
- Shear strengths in columns  $0.7\sum A_c$   
Both in X- and Y-direction  
 $0.7N/mm^2 \times 600mm \times 600mm \times 10$   
 $= 2520000 \text{ N} = 2520 \text{ kN}$
- Shear strength in walls  $2.5\sum A_w$   
X-dir.: 0  
Y-dir.:  $2.5N/mm^2 \times 180mm \times 5400mm \times 2$   
 $= 4860000 \text{ N} = 4860 \text{ kN}$

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## Answer 2

	shear strength	total weight		
	$0.7\sum A_c + 2.5\sum A_w$	$\sum W$		
X-dir.	2520 + 0	< 3456	N.G.	
Y-dir.	2520 + 4860	> 3456	OK	

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## Root3 (Ultimate strength design)

$$\text{Required story shear } Q_{un} \leq \text{lateral strength } Q_u$$

- Required story shear  
 $Q_{un} = D_s \cdot F_e \cdot F_s \cdot Q_i$  ( $C_0=1.0$  or more)  
 $D_s$ : ductility factor (0.3-0.55)  
 $F_e$ : shape fac. (eccentricity in plan)  
 $F_s$ : shape fac.  
(unbalance of stiffness along height)

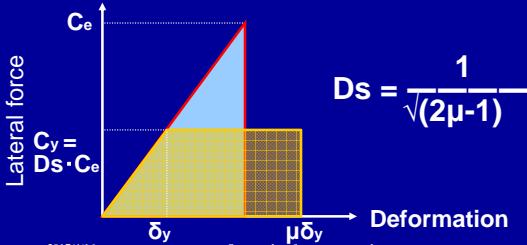
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## Ductility factor $D_s$

- Newmark's law for uniform energy dissipation  
Energy dissipation of an elastic system  $\triangle$  is equal to that of inelastic system  $\square$   
A system with lower strength but large deformation capacity can resist against an EQ.



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## Regulation of $D_s$ values

梁橋の性状	梁橋の形式		
	(イ) 剛接合梁橋又はこれに類する形式の梁橋	(ロ) 欄及び欄に拘束するもの以外のもの	(ハ) 各層に生ずる水平力のうち当該層の耐力又は耐力比を考慮した梁橋
(1) 梁橋を構成する部材に生ずる応力に対しては、新設梁橋に比し、脆性度が高いもの	0.3	0.4	0.45
(2) (1) に拘束するもの以外のもので、梁橋を構成する部材に生ずる応力に対しては、新設梁橋に比し、脆性度が高いもの	0.35	0.4	0.45
(3) (1) 及び(2) に拘束するもの以外のもので、梁橋を構成する部材に生ずる応力に対しては、新設梁橋に比し、脆性度が高いもの	0.4	0.45	0.5
(4) (1) から(3) までのもの	0.45	0.50	0.55

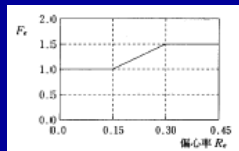
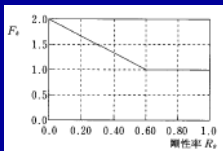
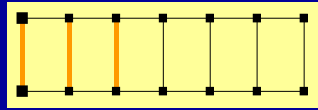
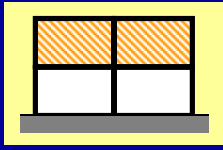
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## Shape factors $F_e$ & $F_s$

- Unbalance of stiffness along building height
- Eccentricity of stiffness in plan



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## Calculation of lateral strength of frame

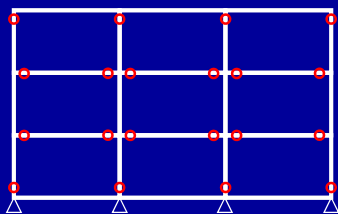
- Cal. of ultimate moment in beam and column.
  - beam:  $M_u = 0.9a_t \cdot \sigma_y \cdot d$
  - column:  $M_u = 0.8a_t \cdot \sigma_y \cdot D + 0.5N \cdot D \cdot (1 - N/bD\sigma_B)$
- Decide hinge location at each node comparing strengths of beams and columns and draw moment diagram.
- Lateral strength can be obtained by summation of shear in all the columns in a story.

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## Lateral strength



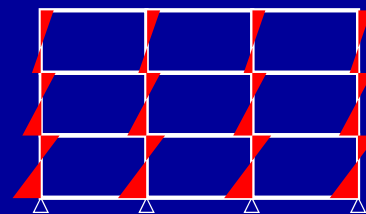
- Compare  $M_u$  in beams and in columns.
- Assume plastic hinge at ends of member with smaller  $M_u$
- Distribute  $M_u$  at plastic hinges to members without hinges

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## Moment diagram in column



- Lateral strength (story shear) can be obtained by summation of shear in all the columns in a story.

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