

# Structural Performance Control Guidance

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## Introduction (1)

### ■ Purpose of structural design

- Chap.20 in Japanese Building Standard Law  
"A building should be consist of safe structure against dead load, live load, snow, wind, soil and water pressure as well as shaking due to earthquake and impact."
- Safety is the only requirement?
  - Building should keep function during life time with maintenance and repair, if necessary
    - ➔ requirement for building structures has been upgraded and diversified with development of modern society.

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## Introduction (2)

- Requirement for building structures
  - Serviceability: maintain function of building
  - Repairability: repairable within reasonable repair costs
  - Safety: save human lives and household effects
- Sustainable building structures
  - effective use of natural resources
  - Reduction of environmental burden

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## Objective of lecture

- To learn basic concept and method for sustainable building structures with required performance against various loads and forces.
- To introduce following items;
  - Structural design method
  - Performance evaluation method
  - Theory of reliability and risk assessment

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## Contents of lecture

- Current seismic design standard
  - Allowable stress design and ultimate strength design
- Seismic evaluation and retrofit for existing buildings
  - Seismic capacity index,  $I_s$
  - Technique and example of seismic strengthening
- New seismic design and performance evaluation
  - Guideline for Earthquake Resistant Building
  - Guideline for Seismic Performance Evaluation
- Reliability analysis and risk assessment
  - Basic of reliability analysis by probabilistic approach
  - Seismic risk and prediction of damage to buildings
  - Concept of seismic risk management

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## Introduction (today's contents)

### 1. History of

**Damage of RC buildings due to earthquake and seismic code**

### 2. Action prior to EQs

**Outline of seismic evaluation and retrofit**

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# 1. History of Damage of RC buildings due to earthquake and seismic code

## Damage and seismic code in Japan

- 1891 Nohbi EQ
- 1923 Kanyto EQ M7.9 approx. 140,000 deaths
- 1924 Revision of Building Standard Law
- 1948 Fukui EQ M7.3 3895 deaths
- 1950 New Building Standard Law
- 1964 Niigata EQ M7.5 26 deaths
- 1968 Tokachi-oki EQ M7.9 52 deaths  
shear failure of columns in RC buildings
- 1971 Revision of seismic code
  - enforcement of requirement for hoop in column

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## Damage and seismic code in Japan

- 1975 Ohita Chubu EQ M6.4 No death
- 1977 Seismic Evaluation Standard
- 1978 Miyagi-ken-oki EQ M7.4 28 deaths
- 1981 Revision of seismic code
  - 2<sup>nd</sup> level design
- 1983 日本海中部地震 M7.7 104 deaths(tsunami)
- 1993 Kushiro-oki EQ M7.8 2 deaths
- 1993 Hokkaido-nansei-oki EQ M7.8 230 deaths  
(tsunami)
- 1994 Hokkaido-toho-oki EQ M8.1 No death
- 1994 Sanriku Far-off EQ M7.5 3 deaths

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## Damage and seismic code in Japan

- 1995 Kobe EQ M7.2 over 6000 deaths
  - ✓ More damage to buildings design by old seismic code
- 1995 Law for Promotion of Seismic Retrofit
  - Seismic retrofit of existing buildings
- 1997 Guideline for Design of Earthquake Resistant Building (AIJ, Architectural Institute of Japan)
- 1999 Revision of seismic code
  - Performance -based
- 2004 Guideline for Performance Evaluation (AIJ)
- 2004 Niigata Chuetsu EQ M6.8 68 deaths
- 2008 Iwate Miyagi EQ M7.2 17 deaths
- 2011 East Japan EQ M9.0 over 20,000 deaths

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## Damage of RC structures by EQ



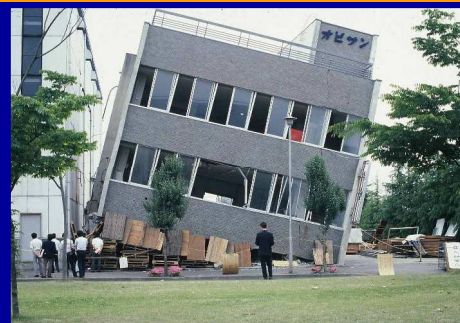
Hakodate Univ. (1968 Tokachi-oki EQ)

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## Damage of RC structures by EQ



Collapse of buildings (1978 Miyagi-ken-oki EQ)

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## Damage of RC structures by EQ



Collapse of buildings (1978 Miyagi-ken-oki EQ)

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## Damage of RC structures by EQ



Collapse of RC buildings (1995 Kobe EQ)

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## Damage of RC structures by EQ



Story collapse (1995 Kobe EQ)

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## Damage of RC structures by EQ



Story collapse (1995 Kobe EQ)

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## Damage of RC structures by EQ



Shear failure (1995 Kobe EQ)

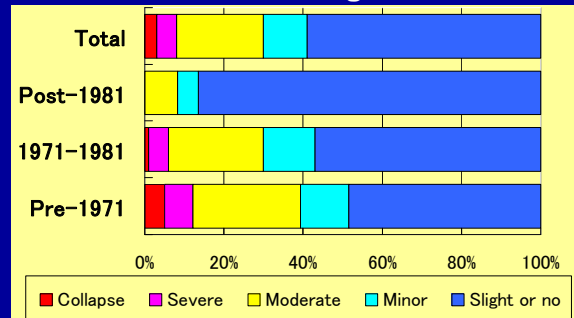
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## Damage statistics (Kobe EQ)

### 631 RC school buildings

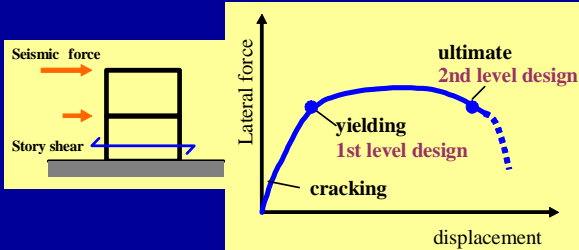


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## Seismic design in Building Law (1)



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## Seismic design in Building Law (2)

### ■ Allowable stress design (1<sup>st</sup> level design)

$$\begin{aligned} \text{Seismic force} &= \text{weight} \times \text{acceleration response} \\ &= m \times 0.2g \\ &= 0.2W \quad (W: \text{weight of building}) \end{aligned}$$

Target of design:

Prevent severe damage against moderate earthquake  
(serviceability, reparability)

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## Seismic design in Building Law(3)

### ■ Ultimate strength design

(2<sup>nd</sup> level design, after 1981)

Seismic force

$$= 1.0W \times \text{structural characteristic coef. } D_s$$

$$\text{RC: } D_s = 0.3-0.55$$

in accordance with inelastic deformation capacity

Target of design:

prevent collapse against large earthquake (safety)

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## 2011 East Japan EQ



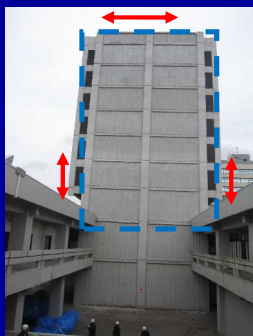
Severely damaged school building (Shitigahama town)  
Shear failure of RC wall

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## 2011 East Japan EQ



Severely damaged building  
(Civil Eng. Building, Tohoku Univ.)

- Upper part of building, especially 3<sup>rd</sup> story, was damaged severely.
- Crush of concrete and buckling/fracture of steel at the bottom of column in 3<sup>rd</sup> story
- Shear wall deformed in north-south direction

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## 2011 East Japan EQ

Failure of column bottom in 3<sup>rd</sup> story of Civil Eng. Building, Tohoku Univ.



Large axial force by Overturning moment in upper part of shear wall induced large axial force in columns.

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### 2011 East Japan EQ

Penthouse of Electrical Building, Tohoku Univ.



RC wall and column failed totally.

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### 2011 East Japan EQ

Damage to non-structural elements

(apartment, Sendai city)

- Const. in 1979
- 11-storied, SRC
- Seismically evaluated



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### 2011 East Japan EQ

Damage to basement

Junior High School, Ohsaki city  
Settlement of 60-70cm, inclination of 1/25 rad.  
Settlement of 20cm, inclination of 1/200 rad.



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### 2011 East Japan EQ

Damage due to tsunami

(K. High School, Kesen-numa city)



Tsunami reached upto 3<sup>rd</sup> floor level.

Debris in building

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### 2011 East Japan EQ

Damage due to tsunami

(Ohkawa prim. School, Ishinomaki city)



Cantilever column of bridge collapsed.



Floor slab was uplifted and connection with beam fractured.

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### 2011 East Japan EQ

Damage due to tsunami

Onagawa, Miyagi pref.



Overturned building

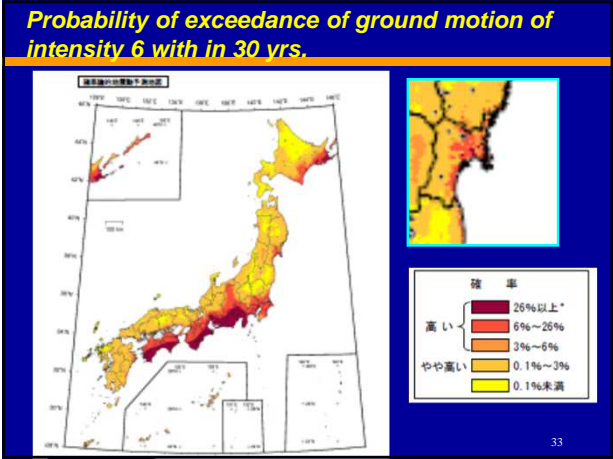
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## 2. Seismic capacity and damage of buildings



- What is seismic evaluation?**
- Building by previous seismic code (-1981)
    - Seismic force of 0.2W → escape damage
    - Unclear against larger EQ
  - Building by current seismic code (1981-)
    - Seismic force of 0.2W → escape damage
    - Seismic force of 1.0W → prevalent collapse
    - Unclear against larger EQ

Seismic capacities of buildings vary in their structural characteristics.
  - Numerical Evaluation of seismic capacity
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- Low for promotion of seismic upgrade**
- Promotion of seismic evaluation and upgrade for existing building by previous seismic code
  - Target: public buildings such as school, hospital, department store, supermarket and hotel
  - Improve capacity up to the level in current code
  - "Seismic Evaluation Standard" is applied.
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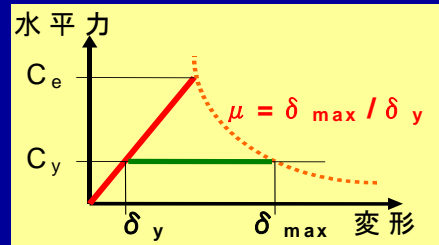
- Seismic capacity index  $I_s$**
- $I_s = E_0 \times S_D \times T$ 
    - $E_0$  : basic capacity index  
= C(strength) × F(ductility)
    - $S_D$  : shape index  
→ irregularity of plan and elevation
    - $T$  : age index  
→ deterioration after construction
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### Basic capacity index $E_0$

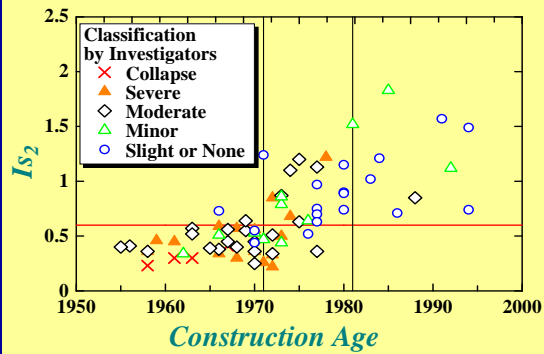
- $E_0 = \varphi \times C \times F$   
 = lateral force  $\times$  deformation  
 $\varphi$ : modification factor for vibration mode  
 $C$ : strength index  
 = lateral strength  $Q_u$  / building weight  $W$   
 (story shear coefficient)  
 $F$ : ductility index

### Characteristics of seismic index $I_s$

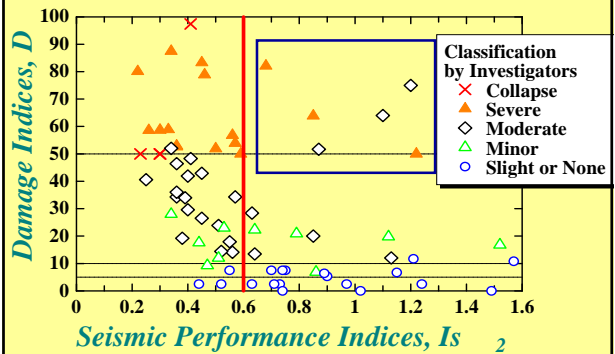
- Seismic capacity of a building is evaluated by
  - Lateral strength and ductility



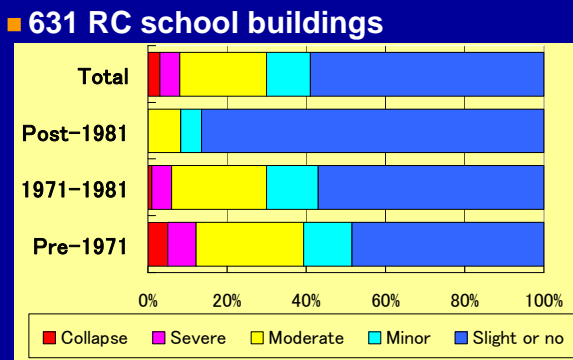
### $I_s$ -index and construction age



### $I_s$ -index and damage level



### Damage statistics (kobe EQ)



### Judging criteria

$$I_s \geq I_{s0}$$

$$C_T \cdot S_D \geq 0.3$$

- Requirement  $I_{s0} = E_s \cdot Z \cdot G \cdot U$   
 $E_s = 0.8$  (1<sup>st</sup> level),  $0.6$  (2<sup>nd</sup> level)  
 $Z$ : zone factor (0.7-1.0)  
 $G$ : ground factor (1.0, 1.25)  
 $U$ : usage factor (1.0, 1.1, 1.2)

#### Lateral strength index

$$C_T = (n+1)/(n+i) \cdot (C_1 + \alpha_2 C_2 + \alpha_3 C_3)$$

## Is index and requirement in seismic code

- Requirement of lateral strength in current code

$$Q_{un} = D_s F_{es} A_i Z R_t C_0 \Sigma W$$

below is a variation of expression

$$C_0 = \frac{1}{A_i} \times \frac{Q_{un}}{\Sigma W} \times \frac{1}{D_s} \times \frac{1}{F_{es}} \times \frac{1}{Z R_t}$$

Vibration mode  
Story shear coef.  
ductility  
Shape index

$$I_s = \varphi \times C \times F \times S_D \times T$$

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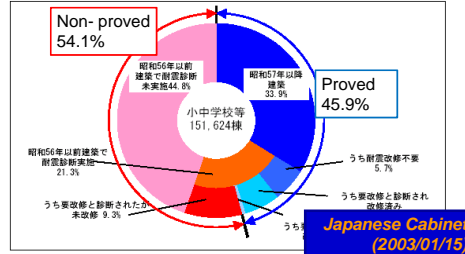
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## Progress of seismic retrofit (2003)

### Seismicity of school buildings in Japan

・本調査において回答のあった小中学校等50,931施設(151,624棟)における耐震化率は約46%。



Japanese Cabinet office  
(2003/01/15)

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## Progress of seismic retrofit (2011)

平成23(2011)年度公立学校施設の耐震改修状況調査による耐震化の状況(小中学校) 資料2

